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JOB SUNY Buffalo Bridge

SHEET NO. _____ OF _____

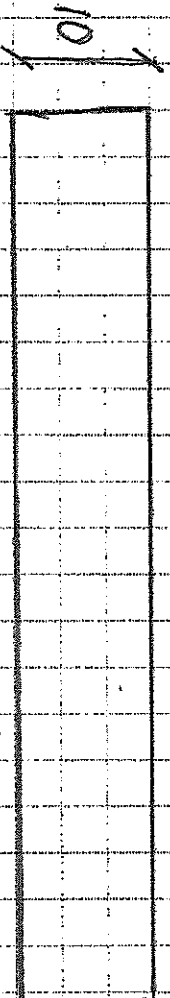
CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____

Live load 100 psf

84



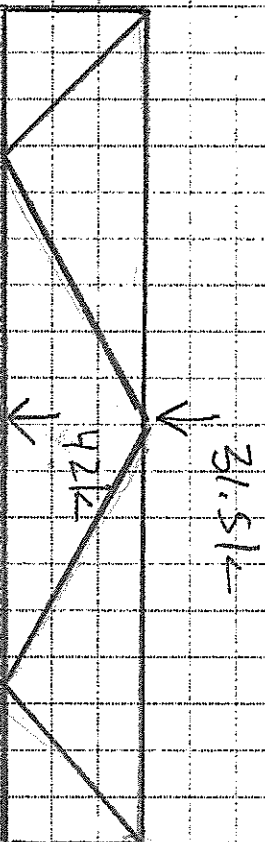
$$10(84) = 840 \text{ ft}^2 \quad (100 \text{ psf}) = 84 \text{ k}$$

Snow load

$$P_t = .7 C_e C_d I P_g = 7$$

Assume 50 psf snow load

$$15(84) = 1260(50) = 63 \text{ k}$$



Live + Dead load on one side



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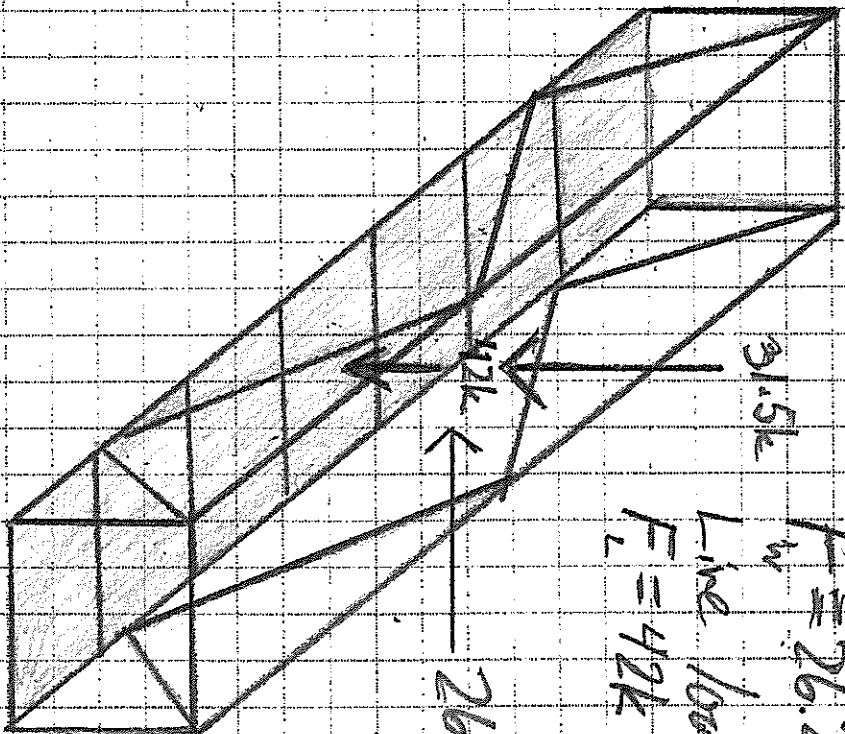
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Wind Load Snow Load

$$F_w = 26.2 \quad F_s = 31.5k$$

Line Load
 $F_L = 42k$

Wind Load

$$F = q_z G C_e A_e = q_z =$$

$$G = .85 (\text{rigid}) \quad C_e = 1.33 \quad A_e = 1176 \text{ ft}^2$$



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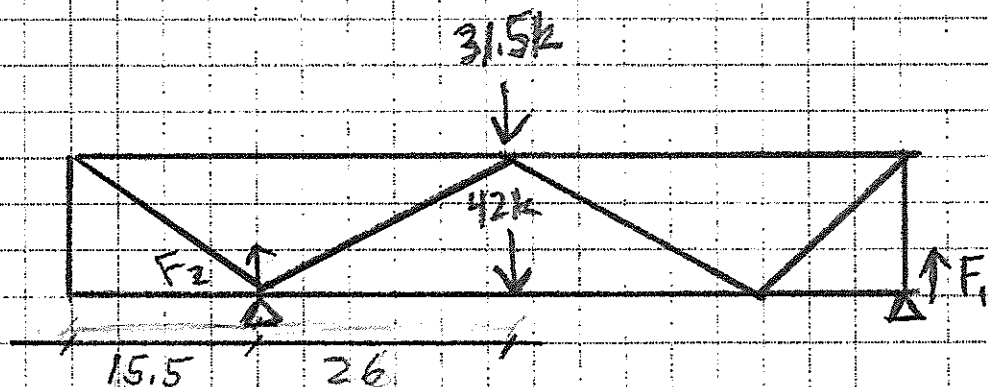
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$$\sum F_y = 0$$

$$F_1 + F_2 = 73.5$$

$$\sum M = 0$$

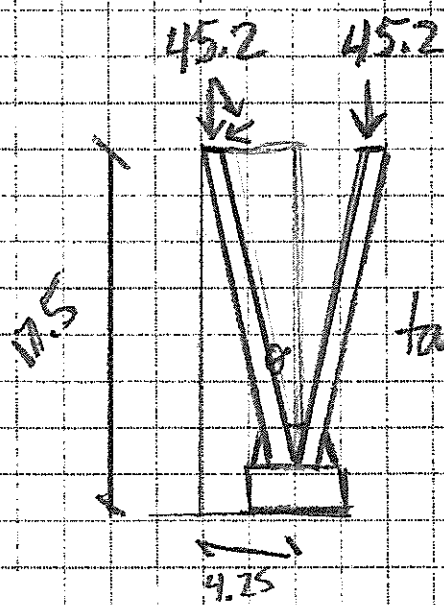
$$(41.5)(73.5) = (67.5)(F_2)$$

$$3050 = 67.5 F_2$$

$$F_2 = 45.2$$

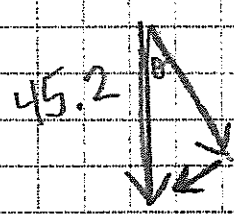
$$F_1 = 28.31$$

$$F_2 = 90k \text{ at columns}$$



$$\tan \theta = \frac{4.25}{17.5}$$

$$\theta = 13.65^\circ$$



$$\sin 13.65 = \frac{x}{45.2} \quad x = 10.67$$

$$\cos 13.65 = \frac{y}{45.2} \quad y = 43.92$$



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JOB 5144 Btalo Bridge

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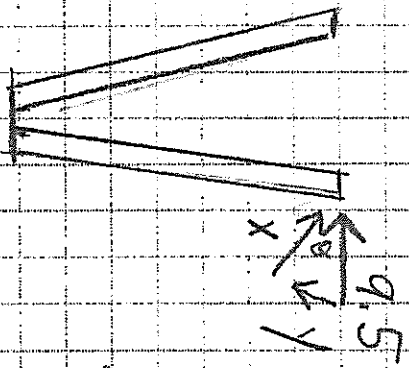
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Moment on Column from Wind

$$ZF_w = 26.2k$$

$\approx 19k$ at column

support



$19/2 = 9.5k$ each column

$$R = 13.65$$

$$\sin 13.65 = \frac{y}{9.5} \quad y = 2.24$$

$$\cos 13.65 = \frac{x}{9.5} \quad x = 9.23$$

total forces on each column

$$X = 9.23 + 10.67 = 19.9 \quad Y = 2.24 + 43.92 = 46.16$$

$$\text{axial} = 46.16k$$

$$\text{moment} = 19.9(18) = 358.41 \text{ ft}$$

Try W14 x 159

$$\frac{P_c}{P_c} + \frac{M_c}{M_c} \leq 1.0$$

$$\frac{46.16}{318} + \frac{358}{716} =$$

$$.15 + .5 = .65$$



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$$q_z = .00256 K_z K_{zt} K_d V^2 I$$

$$K_{zt} = 1 \quad K_z = .68 \quad K_d = .85 \quad I = 1.1$$

$$V = 110$$

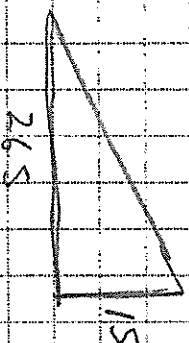
$$q_z = .00256 (.68) (1) (.85) (110)^2 (1.1)$$

$$q_z = 19.69$$

$$F = (19.69) (.85) (1.33) (1176) = 26.2K$$

Deflection Check

brace HSS 6x6x5/8

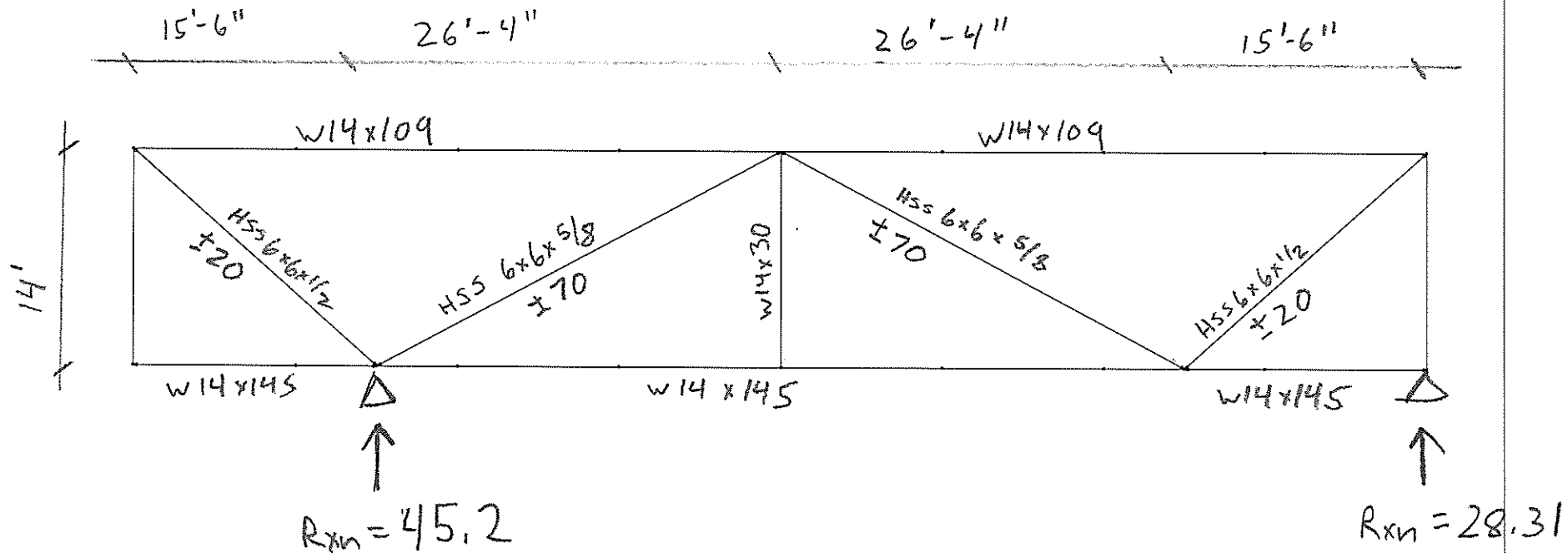


$$L/120 = 30.5/120 = 3.04''$$

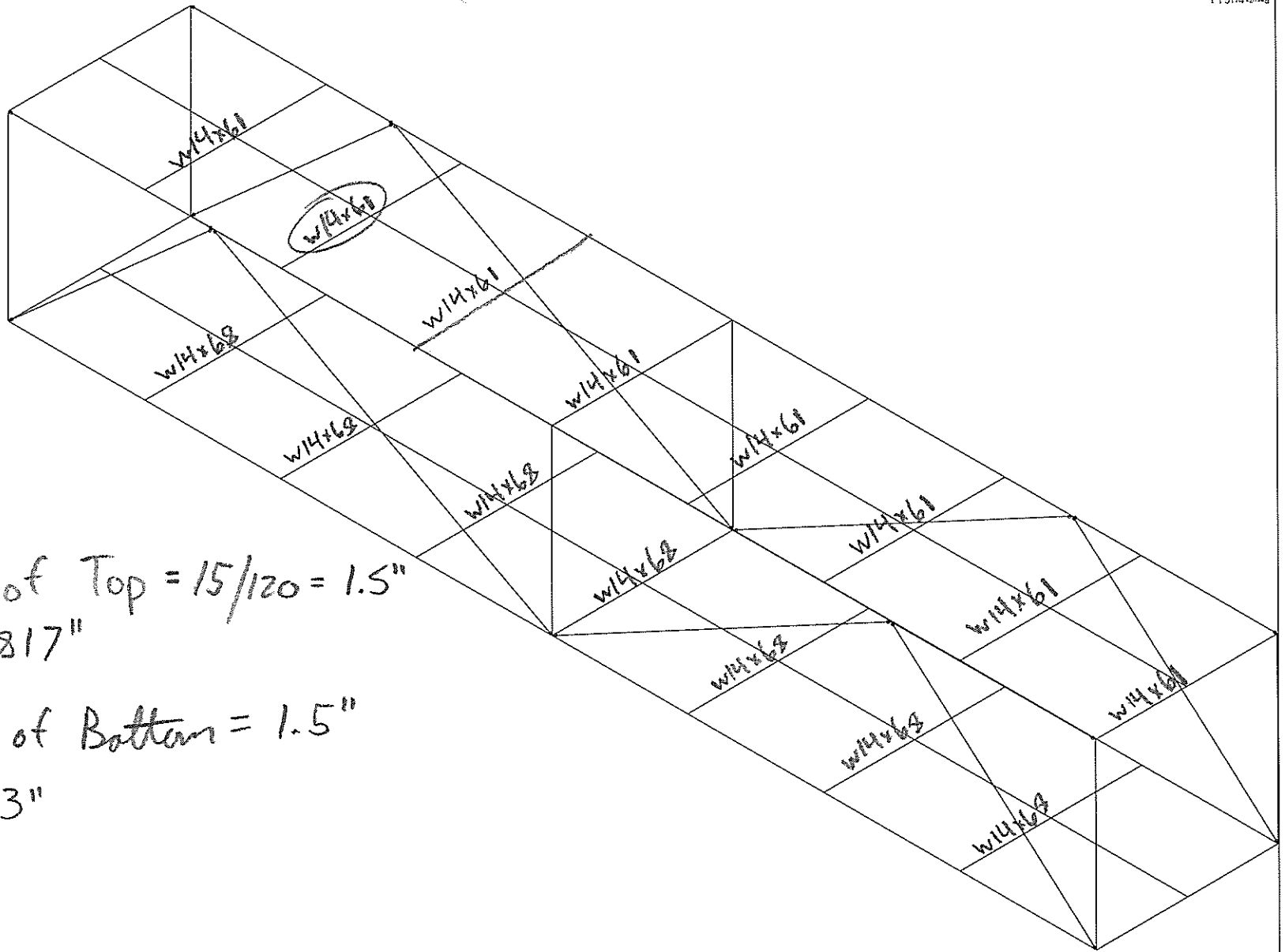
$$\sqrt{26.5^2 + 115^2} = L = 30.5$$

6k

Loads = Live = 100 psf
 Dead = 70 psf
 Snow = 50 psf
 Wind = 25 psf



Deflection of bottom chord = .5" $L/120 = 50/120 = 5"$
 Deflection of top chord = .8" $L/120 = 42/120 = 4.2"$
 Deflection of brace = .8" $L/120 = 30/120 = 3"$



$$\text{Deflection of Top} = 15/120 = 1.5'' \\ = .817''$$

$$\text{Deflection of Bottom} = 1.5'' \\ = .3''$$

